

# Effect on Bituminous Mix Pavement using Natural Fiber

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## Abstract

For the most part a bituminous combination is a combination of coarse total, fine total, filler and folio. A Hot Mix Asphalt is a bituminous blend where all constituents are blended, set and compacted at high temperature. HMA can be Dense Graded blends (DGM) known as Bituminous Concrete (BC) or hole reviewed known as Stone Matrix Asphalt (SMA). SMA requires settling added substances made out of cellulose fibbers, mineral strands or polymers to forestall channel down of the blend. It is presumed that expansion of sisal fiber further develop the blend properties like Marshall Stability, Drain down attributes and circuitous rigidity in the event of both BC and SMA blends. It is seen that SMA is superior to BC in regard of backhanded elasticity and creep qualities.

**Keywords:** Bituminous Concrete (BC), Stone Matrix Asphalt (SMA), Sisal Fiber, Marshall Properties, Static Indirect Tensile Strength, Static Creep.

## I. Introduction

Parkway development includes gigantic cost of venture. "A particular designing plan might save significant venture as well as solid execution of the in-administration parkway achieved. Things which are of significant contemplations in adaptable asphalt designing asphalt plan and the blend plan. The current review is worried to the blend plan contemplations.

A decent plan of bituminous blend in asphalt is supposed to bring about a blend which is sufficiently major areas of strength for i sturdy (iii) resistive to weakness and long-lasting twisting (iv) climate cordial (v) practical, etc. A blend configuration arranged necessities to accomplish these prerequisites through various tests on the blend in with fluctuated extents and concludes with the best one. The current examination work plans to recognize a portion of the issues engaged with this specialty of bituminous blend plan and the bearing of the exploration.

A blend that comprise of totals consistently reviewed from most extreme size, regularly under 25 mm, through the fine filler that is more modest than 0.075 mmis called Asphaltic/Bituminous cement. Adequate amount of bitumen is included with the blend so the compacted blend

is really impenetrable and will have OK dissipative and flexible properties. The bituminous blend configuration targets deciding the extent of bitumen, filler, fine totals, and coarse totals to create a blend which is, areas of strength for functional, and economical Highway development includes enormous cost of venture. A particular designing plan might save impressive venture as well as dependable execution of the in-administration expressway achieved. Things which are of significant contemplations in adaptable asphalt designing asphalt plan and the blend plan. The current review is worried to the blend plan contemplations.

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A blend that comprise of totals persistently evaluated from most extreme size, commonly under 25 mm, through the fine filler that is more modest than 0.075 mmis called Asphaltic/Bituminous cement. Adequate amount of bitumen is included with the blend so the compacted blend is really impenetrable and will have OK dissipative and flexible properties. The bituminous blend configuration targets deciding the extent of bitumen, filler, fine totals, and coarse totals to deliver a blend which is, areas of strength for useful. The main aim of the mix design is to produce a bituminous mix by proportioning various components so as to have-

1. Sufficient bitumen to durable pavement
2. Sufficient strength to resist shear deformation under traffic at high temperature
3. Sufficient workability, to permit easy placement without segregation
4. Sufficient resistance to avoid premature cracks occurred

due to repeated bending by traffic

5. Sufficient resistance at low temperature to prevent shrinkage cracks.

## II. Objective

A comparative study has been made in this research work between Bituminous Concrete (BC) and Stone Matrix Asphalt (SMA) mixes with varying binder contents (4% - 7%) and Fibre contents (0.3% - 0.5%). In the present study of mix 60/70 penetration grade bitumen is used as binder and Sisal fibre is used as stabilizing additive.

## III. Review of Literature

Asphalt comprises of more than one layer of various material laid on a layer called sub grade. For the most part there is two sorts of asphalt - adaptable asphalt and Rigid asphalt. Adaptable asphalts are so named on the grounds that, the complete asphalt structure redirects or flexes under stacking. An adaptable asphalt structure is ordinarily comprise of a few layers of material. Each layer gets the heaps from the above layer which spreads them out, then, at that point, gives these heaps to the following layer beneath. Regular adaptable asphalt structure made out of the accompanying:

- *Surface course.* This is the top most layer and this layer comes in contact with traffic. It may consist of one or several different HMA sub layers. HMA is a mixture of fine and coarse aggregates and asphalt binder
- *Base course.* This is the layer directly below the HMA layer and generally composed of aggregate (either stabilized or run-stabilized).
- *Sub-base course.* This is the layer (or layers) under the base layer, sub-base is not always required.

### Stabilizing Additives:-

Stabilizing additives are used in the mix to prevent mortar drain down and to provide better binding. Fibres commonly used at present are polypropylene, polyester, mineral and cellulose. The main stabilizing additives used in mixes can be classified into different groups as follows;

- Fibers' (Cellulose Fibers, Chemical Fibers, Mineral Fibers)
- Polymer
- Plastics (Polymer Powders or Pellets)
- Powder and flour like materials (Silica acid, Special Filler)

## IV. Experimental Methodology

This chapter describes the experimental work that was done in this study. This chapter is divided into two sections. The first portion covers component experiments (aggregates, filler, bitumen, and fibre), whereas the second segment covers bituminous mix trials.

### Preparation of Mixes

The combinations were made by ASTM D1559. The Marshall method was utilized. The coarse totals, fine totals, and filler for BC and SMA were blended by the degree embraced, as displayed in Tables 3.1 and 3.2, separately. Initially, an examination research on BC is directed utilizing three particular sorts of filler: concrete, fly debris, and stone residue. The Marshall Test found the Optimum Binder Content (OBC), which goes from 0% to 7% fastener content. The Marshall Method was then used to decide the Optimum Binder Content (OBC) and Optimum Fiber Content (OFC) of both BC and SMA, with cover content going from 0% to 7% and fiber content going from 0.3 to 0.5 percent. Subsequent to being cut into little pieces (15-20 mm), the sisal strands were added to the total example in different sums. Independently, the mineral totals with strands and folios were warmed to the blending temperature. The temperature of the mineral totals was kept 10 degrees Celsius higher than the temperature of the cover. The expected measure of fastener was added to the preheated total fiber blend, and the combination was all around blended by hand until the variety and consistency seemed to be predictable. The blending term was kept somewhere in the range of 2 and 5 minutes. The fluid was then positioned into Marshall Molds that had been pre-warmed, and the examples were ready with a comp active exertion of 75 blows on each side. The examples were passed on to cool to room temperature short-term. From that point forward, the examples were removed and investigated at 60°C, according to typical strategy.

### Test son Mixes

The various tests on bituminous mixes with varying binder type and quantity, as well as fiber concentration in the mix, are presented below.

**Marshall Test-** Marshall Mix design is a standard laboratory method for measuring and reporting the strength and flow characteristics of bituminous pavement mixes that is used all over the world. It is a widely used method of characterization of bituminous mixtures in India.

**Drain down test-** The drain-down characteristics of bituminous mixtures can be evaluated using a variety of approaches. In this investigation, the drain down approach

proposed by MORTH (2001) was used”.

Drain down equation is

$$d = \frac{W_2 - W_1}{1200 + X}$$

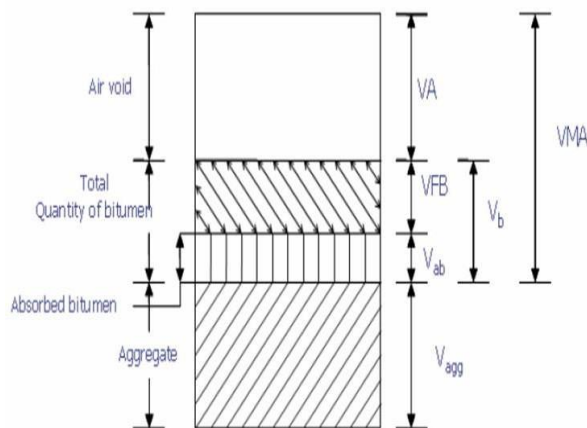
Where,

$W_1$  = initial mass of the plate

$W_2$  = final mass of the plate and drained binder  $X$  = initial mass of fiber in the mix

“Three mixtures were made at the optimum binder content for each binder, and the drain down was calculated as the average of the three.

**Backhanded Tensile Strength Test**-The aberrant elasticity (ITS) of bituminous blends is resolved utilizing a circuitous ductile test. A compressive power is given to a round and hollow example (Marshall Sample) in this test along an upward polar plane by means of two bended strips with a similar span of ebb and flow as the example.



**Figure1: Phase Diagram of bituminous mix**

**Static Indirect Tensile Test**-The Marshall test contraption was utilized for this test, which had a deformity pace of 51 mm each moment. A compressive burden was applied in the upward polar plane, and the heap was estimated with a demonstrating ring. To keep the testing temperature consistent, a Perspex water shower (270 mm x 250 mm x 195 mm) was made. Two hardened steel stacking strips, 13 mm (1/2) wide, 13 mm profound, and 75 mm long, were used to move the applied burden to the example. The strip fabricated has a similar inside width as a Marshall test (102 mm). The static roundabout elastic test on an example is displayed in Figure 3.5. A nearby of the stacked example is displayed in Fig. 3.6. Prior to testing, the example was held in a water shower at the fundamental temperature for no less than 1/2 hour. On the base plate of the Marshall

contraption, a Perspex water shower kept at a similar test temperature was introduced. Inside the two stacking strips, the example was kept inside the Perspex water shower. A 51 mm/minute stacking rate was picked. The test temperature was varied in 5°C increments from 5°C to 40°C. Three Marshall samples were tested at a specific temperature in this test, and the tensile strength was calculated as the average of the three values

**Static Creep Test**- Samples for the Static Creep test were made at their OBC and OFC. There are two stages to the test.

A vertical load of 6 KN is applied for 30 minutes in the first stage. The deformation was measured with a dial gauge calibrated in 0.002 mm units and capable of registering a maximum deflection of 5 mm throughout these 0, 10, 20, and 30 minutes. Second, the load was withdrawn, and its deformation was measured over a 10-minute period (40, 50, 60 minutes). The temperature is kept at 40°C during the test.

## V. Result and Analysis

The outcomes and perceptions of the tests led in the past section are given, analyzed, and examined in this part. There are five segments in this part. The main area talks about the boundaries used in the examination. The subsequent segment covers the calculation of BC's Optimum Binder Content (OBC), which incorporates fillers like concrete, fly debris, and stone residue. The third segment examines how to work out Optimum Binder Content (OBC) and Optimum Fiber Content (OFC), as well as Marshall Properties of BC with and without fiber. The fourth area examines how to work out the Optimum Binder Content (OBC) and Optimum Fiber Content (OFC), as well as the Marshall Properties of SMA regardless of fiber. The fifth area examines the consequences of the Drain Down and Static Indirect Tensile Stress tests, as well as the static Creep test”.

### Optimum Binder content

“The optimal binder content is determined by averaging the following three bitumen content values given in the graph above:

- Bitumen content corresponds to the highest level of stability
- Bitumen content correspond to maximum unit weight
- Bitumen content equal to the median of the prescribed limitations for percentage air voids in total mix

**Table 1.OBC of SMA with different fibre Content**

BC With fiber content (%)	OBC (%)
0	5.8
0.3	5.2
0.4	6.2

As a result, the OBC for SMA is 5.2 percent and the OFC is 0.3 percent. In comparison to mixes containing 0.4 percent fibre, it can be shown that adding 0.3 percent fibre increases not only the stability value but also the binder quantity. If the binder content is high, the binder in the mix will drain. As a result, the OFC for SMA is 0.3 percent.

## VI. Conclusion

SMA and BC blends are made here, with 60/70 infiltration grade bitumen filling in as the fastener. A normally happening fiber known as sisal fiber is likewise used in differed focuses (0 to 0.5 percent ). Marshall Method of Mix Design is utilized to decide OBC and OFC. Blend qualities are for the most part improved by adding 0.3 percent fiber. SMA with sisal fiber creates very great outcomes and can be used in adaptable asphalt, as per a few tests like channel down test, backhanded rigidity”, and static wet blanket test.

## References

- [1] "Test Method for Resistance to Plastic Flow of Bituminous Mixtures Using Marshall Apparatus," ASTM D 1559 (1989).
- [2] "Roundabout Tensile (IDT) Strength for Bituminous Mixtures," ASTM D 6931 (2007).
- [3] "Assessment of Laboratory Properties of SMA Mixtures," NCAT Report No. 93-5, Auburn University, Alabama, 1993. Brown, E.R., and Manglorkar, H
- [4] "Stone Matrix Asphalt Properties Related to Mixture Design," NCAT Report 94-02, E.R. Brown and R.B. Mallick, 1994.
- [5] "Squander Plastic as a Stabilizing added substance in SMA," International Journal of Engineering and Technology, Volume 2, Issue 6, pages 379-387, C.S. Bindu and Beena K.S.
- [6] Prentice Hall of India, New Delhi, pp 294-299, Das A. furthermore, Chakroborty P. (2010), "Standards of Transportation Engineering.
- [7] Das A., Deol, M. S. Ohri, S. furthermore, Pandey B. B.(2004). "Development of non-standard bituminous blend - a concentrate on Indian determination", The International Journal of Pavement Engineering, Vol 5(1), pp. 39-46.
- [8] H. Jony Hassan, Y. JahadIsraa(2010),“The Effect of Using waste Glass control filler on Hot Asphalt Concrete Mixture Properties”, EnggandTechnologyjournal,vol.29,Issue1,pp44-57
- [9] <http://www.pavementinteractive.org/article/hma-performance-test/>
- [10] <http://www.pavementinteractive.org/article/hma-types/>  
J.V. Merighi, R.M. Fortes, "The Study of long-lasting distortion utilizing static drag test.
- [11] "Use of Waste Fiber in Stone Matrix Asphalt Mixtures," by Bradley J. Putman and Serji N. Amirkhanian, Resources, Conservation and Recycling, Volume 42, Issue 3, pages 265-274, 2004.
- [12] "Stone Mastic Asphalt (SMA) - A Long Life Pavement Surface," International Seminar on Innovations in Construction and Maintenance of Flexible Pavements, Agra, 2-4 September, Technical Papers, Volume 1, pages 169-17, Bose S., Kamaraj C., and Nanda P.K
- [13] "A Laboratory Investigation on Stone Matrix Asphalt using Ground Tire Rubber," Chui-Te Chiu and Li-Cheng Lu (2007), Construction and Building Materials, Volume 21, Issue 5, pages 1027-1033.